

Causeway Coast and Glens Borough Council Local Planning Office Cloonavin 66 Portstewart Road, Coleraine BT52 1EY

26 August 2025

Our ref: M01616-36

Planning Ref LA01/2025/0181/F - 340m South of 5 Magheraboy Road, Rasharkin

Re: Drainage Assessment- Dfl Rivers Consultation / Policy FLD3

To whom it concerns,

McCloy Consulting provided the existing Drainage Assessment which accompanied the above referenced planning application.

This submission sets out further work undertaken to address the issues expressed by Dfl Rivers in its consultation dated 4 June 2025. Rivers in consultation has expressed no concern in relation to Policies FLD1, 2, 4 and 5 of Revised PPS15. Rivers has requested clarifications in relation to Policy FLD3.

This submission is supplementary to the information in the existing Drainage Assessment.

#### Nature of the Clarification

The consultation response requests that it is demonstrated that the total post-development runoff (including the greenfield run-off that is not positively drained in the development's drainage network) which discharges into the undesignated watercourse that runs along the Eastern and northern boundaries of the site, does not exceed the original greenfield run-off in order to prove that there is no increase in flood risk elsewhere as a result of the development.

The response has arisen as a result of the site being affected by a split catchment, where an area of c. 1.63Ha drains south-westerly and an area of c. 4.51Ha drains northeasterly. Dfl Rivers Directorate request demonstration that flow rates to the north-easterly watercourse from the proposed site outfall do not exceed the pre-development greenfield rate of discharge from the site development area to this watercourse.

As such we attach an updated Drainage Plan and Microdrainage Calculations which demonstrate that flows draining to the north-eastern watercourse do not exceed the pre-development flow rates for the north-eastern catchment area only. The below table and figure included in Annex A offers explanation to the updated flow rates. Note that the drainage regime to undeveloped areas within the site shall not undergo changes which will affect run-off rates and the proposed access track shall remain a permeable surface, as such these areas are not considered in Table 1.



Table 1 Catchment Analysis and Assigned Run-off Rates

	Total Development Area	North / eastern catchment development area	Southern catchment development area	
Overall area being drained	1.784 Ha	1.061Ha	0.724Ha	
Allowable run-off rate (based on 10lps/Ha)	17.8lps	10.6lps	7.2lps	
Run-off rate applied to design	10.6lps	10.6lps	-	

#### **Summary**

The 10.6lps shown on the revised Drainage Plan and associated Microdrainage Calculations has been adopted so that the total discharge from the development area post-development shall be no greater than the equivalent pre-development greenfield rate to the north-eastern catchment area.

There shall be no discharge to the southern catchment from the developed area post-development, which offers a reduction from the current scenario where a greenfield rate of run-off would apply from the area currently draining south.

#### Conclusion

The information in this memorandum should allow DfI Rivers to conclude that the proposals are compliant with FLD3 as run-off to either watercourse catchment post-development does not exceed pre-development greenfield rates.

Dfl Rivers has been supplied this information informally and any further concerns raised regarding the information supplied have been addressed. Please therefore arrange for Dfl Rivers to be re-consulted in relation to this clarification.

Should you have any queries please do not hesitate in contacting this office.

Yours faithfully

Jill Dick

On behalf of McCloy Consulting

#### Encs:

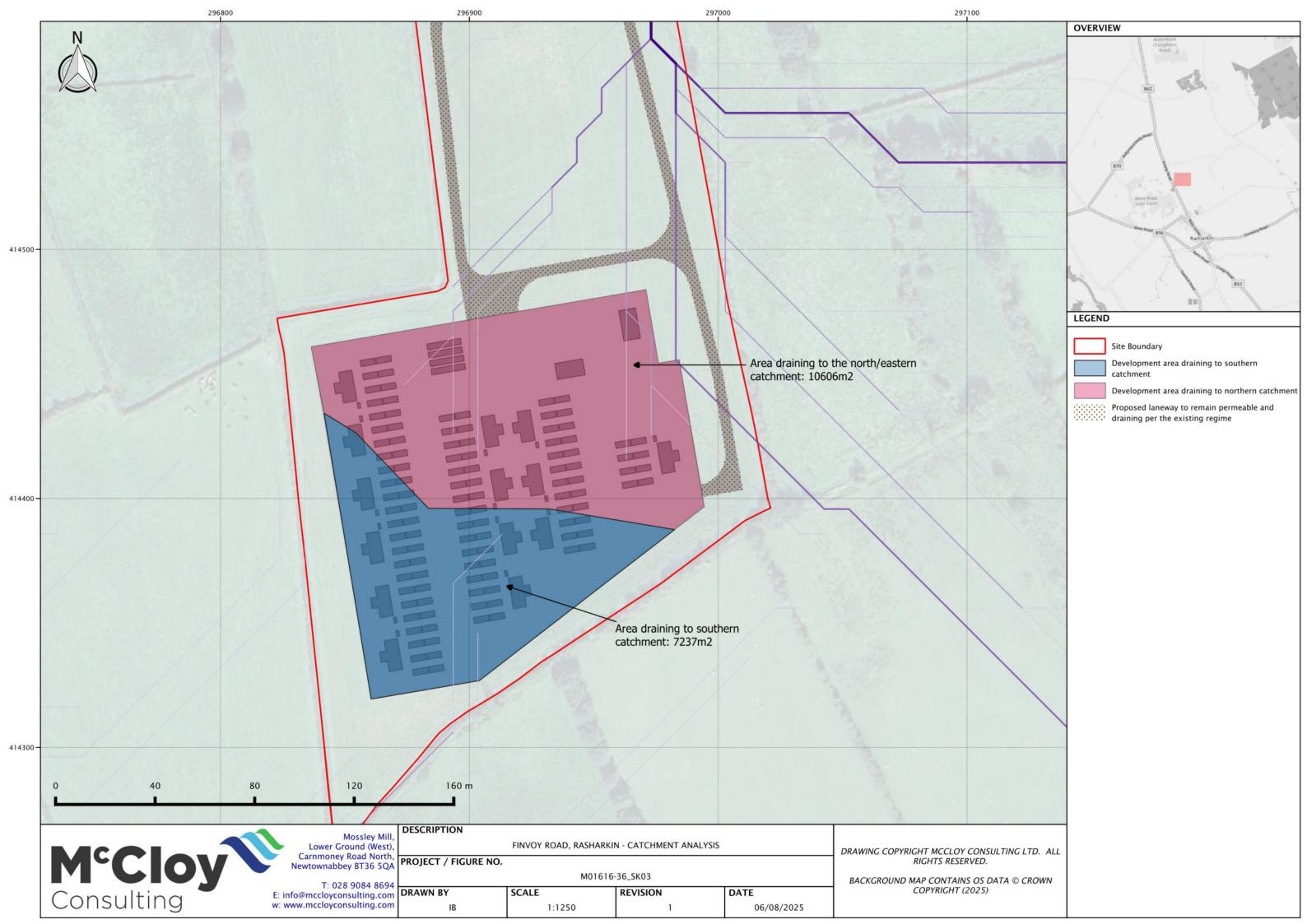
ANNEX A - M01616-36 SK03 Catchment Analysis Figure

**ANNEX B - Microdrainage Calculations** 

ANNEX C -M01616-36 DWG100-03 Revised Drainage Plan



# ANNEX A – M01616-36 SK03 Catchment Analysis Figure





# ANNEX B - Microdrainage Calculations

McCloy Consulting Limited		Page 0
Mossley Mill	M01616-36 Storm Rev 2	
Newtownabbey	Machaire BESS	
Co. Antrim		Micro
Date 10/06/2025 16:58	Designed by JD	Drainage
File M01616-36 MD new layout R2.mdx	Checked by IB	חומוומלה
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#### STORM SEWER DESIGN by the Modified Rational Method

#### Design Criteria for Surface Network 2

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - Scotland and Ireland

Return Period (years) 2 Foul Sewage (1/s/ha) 0.000 Maximum Backdrop Height (m) 0.000 M5-60 (mm) 17.200 Volumetric Runoff Coeff. 0.750 Min Design Depth for Optimisation (m) 1.200 Ratio R 0.288 PIMP (%) 100 Min Vel for Auto Design only (m/s) 1.00

Maximum Rainfall (mm/hr) 50 Add Flow / Climate Change (%) 0 Min Slope for Optimisation (1:X) 500

Maximum Time of Concentration (mins) 30 Minimum Backdrop Height (m) 0.000

Designed with Level Soffits

#### Network Design Table for Surface Network 2

« - Indicates pipe capacity < flow

PN Length Fall Slope I.Area T.E. Base k HYD DIA Section Type Auto (m) (m) (1:X) (ha) (mins) Flow (1/s) (mm) SECT (mm) Design

#### Network Results Table

PN Rain T.C. US/IL  $\Sigma$  I.Area  $\Sigma$  Base Foul Add Flow Vel Cap Flow (mm/hr) (mins) (m) (ha) Flow (l/s) (l/s) (l/s) (m/s) (l/s) (l/s)

McCloy Consulting Limited		Page 1
Mossley Mill	M01616-36 Storm Rev 2	
Newtownabbey	Machaire BESS	
Co. Antrim		Micro
Date 10/06/2025 16:58	Designed by JD	Drainage
File M01616-36 MD new layout R2.mdx	Checked by IB	Dialilade
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# Network Design Table for Surface Network 2

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)		Base Flow (1/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	17.343	0.162	107.1	0.092	5.00	0.0	0.600	0	375	Pipe/Conduit	0
1.001	60.213	0.301	200.0	0.172	0.00	0.0	0.600	0	375	Pipe/Conduit	ĕ
1.002	60.643	0.303	200.1	0.183	0.00	0.0	0.600	0	375	Pipe/Conduit	ĕ
1.003	40.694	0.203	200.5	0.000	0.00	0.0	0.600	0	375	Pipe/Conduit	ĕ
1.004	31.982	0.364	87.9	0.000	0.00	0.0	0.600	0	375	Pipe/Conduit	ě
2.000	55.076	0.442	124.6	0.180	5.00	0.0	0.600	0	300	Pipe/Conduit	0
2.001	66.702	0.445	149.9	0.110	0.00	0.0	0.600	0	300	Pipe/Conduit	ĕ
3.000	45.973	0.230	199.9	0.159	5.00	0.0	0.600	0	300	Pipe/Conduit	•

# Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (1/s)		Add Flow (1/s)	Vel (m/s)	Cap (1/s)	Flow (1/s)
1.000	50.00	5.17	82.225	0.092	0.0	0.0	0.0	1.75	193.4	12.5
1.001	49.49	5.95	82.063	0.264	0.0	0.0	0.0	1.28	141.1	35.4
1.002	47.00	6.74	81.762	0.447	0.0	0.0	0.0	1.28	141.0	56.9
1.003	45.49	7.27	81.459	0.447	0.0	0.0	0.0	1.28	140.9	56.9
1.004	44.75	7.55	81.256	0.447	0.0	0.0	0.0	1.93	213.6	56.9
2.000	50.00	5.65	82.225	0.180	0.0	0.0	0.0	1.41	99.5	24.4
2.001	47.66	6.52	81.783	0.290	0.0	0.0	0.0	1.28	90.6	37.5
3.000	50.00	5.69	82.300	0.159	0.0	0.0	0.0	1.11	78.4	21.5

McCloy Consulting Limited		Page 2
Mossley Mill	M01616-36 Storm Rev 2	
Newtownabbey	Machaire BESS	
Co. Antrim		Micro
Date 10/06/2025 16:58	Designed by JD	Drainage
File M01616-36 MD new layout R2.mdx	Checked by IB	Dialilade
Innovyze	Network 2019.1	<u>'</u>

# Network Design Table for Surface Network 2

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	ıse (1/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
3.001	43.191	0.216	200.0	0.115	0.00	0.0	0.600	0	300	Pipe/Conduit	•
2.002	29.514	0.371	79.6	0.063	0.00	0.0	0.600	0	300	Pipe/Conduit	•
4.000	58.075	0.387	150.1	0.116	5.00	0.0	0.600	0	225	Pipe/Conduit	•
1.006	17.277 54.534 27.505 4.461	0.365	149.4	0.000 0.000 0.000 0.000	0.00 0.00 0.00	0.0	0.600 0.600 0.600	0 0	375 375	Pipe/Conduit Pipe/Conduit Pipe/Conduit Pipe/Conduit	<b>0</b>

# Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	$\Sigma$ Base Flow (1/s)		Add Flow (1/s)	Vel (m/s)	Cap (1/s)	Flow (1/s)
3.001	48.22	6.34	82.070	0.274	0.0	0.0	0.0	1.11	78.3	35.8
2.002	46.83	6.80	81.338	0.627	0.0	0.0	0.0	1.76	124.7	79.5
4.000	49.63	5.91	82.200	0.116	0.0	0.0	0.0	1.07	42.3	15.6
1.005	44.25 42.76		80.892 80.777	1.190	0.0	0.0	0.0		163.0 163.5	• ·
1.007	42.53	8.45	80.714	1.190	0.0	0.0	0.0	4.77	526.9	142.7
1.008	42.37	8.52	79.650	1.190	0.0	0.0	0.0	1.07	42.5«	142.7

McCloy Consulting Limited		Page 3
Mossley Mill	M01616-36 Storm Rev 2	
Newtownabbey	Machaire BESS	
Co. Antrim		Micro
Date 10/06/2025 16:58	Designed by JD	
File M01616-36 MD new layout R2.mdx	Checked by IB	Drainage
Innovyze	Network 2019.1	

#### Network Design Table for Surface Network 2

PN Length Fall Slope I.Area T.E. Base k HYD DIA Section Type Auto (m) (m) (1:X) (ha) (mins) Flow (1/s) (mm) SECT (mm) Design

1.009 29.443 0.196 150.2 0.000 0.00 0.0 0.600 o 225 Pipe/Conduit

#### Network Results Table

PN Rain T.C. US/IL Σ I.Area Σ Base Foul Add Flow Vel Cap Flow (mm/hr) (mins) (m) (ha) Flow (1/s) (1/s) (1/s) (m/s) (1/s) (1/s)

1.009 41.35 8.99 79.620 1.190 0.0 0.0 0.0 1.06 42.3« 142.7

McCloy Consulting Limited		Page 4
Mossley Mill	M01616-36 Storm Rev 2	
Newtownabbey	Machaire BESS	
Co. Antrim		Micro
Date 10/06/2025 16:58	Designed by JD	Drainage
File M01616-36 MD new layout R2.mdx	Checked by IB	pialiade
Innovyze	Network 2019.1	·

MH Name	MH CL (m)	MH Depth (m)	Conr	MH nection	MH Diam.,L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
S1	83.250	1.025	Open	Manhole	1800	1.000	82.225	375				
S2	83.250	1.187	Open	Manhole	1800	1.001	82.063	375	1.000	82.063	375	
s3	83.250	1.488	Open	Manhole	1800	1.002	81.762	375	1.001	81.762	375	
S4	83.250	1.791	Open	Manhole	1800	1.003	81.459	375	1.002	81.459	375	
S5	82.730	1.474	Open	Manhole	1800	1.004	81.256	375	1.003	81.256	375	
S6	83.250	1.025	Open	Manhole	1800	2.000	82.225	300				
s7	83.250	1.467	Open	Manhole	1800	2.001	81.783	300	2.000	81.783	300	
S8	83.250	0.950	Open	Manhole	1800	3.000	82.300	300				
S9	83.250	1.180	Open	Manhole	1800	3.001	82.070	300	3.000	82.070	300	
S10	83.250	1.912	Open	Manhole	1800	2.002	81.338	300	2.001	81.338	300	
									3.001	81.854	300	516
S11	83.250	1.050	Open	Manhole	1500	4.000	82.200	225				
S12	83.000	2.108	Open	Manhole	1800	1.005	80.892	375	1.004	80.892	375	
									2.002	80.967	300	
									4.000	81.813	225	771
S13	83.250	2.473	Open	Manhole	1800	1.006	80.777	375	1.005	80.777	375	
S14	83.250	2.838	Open	Manhole	1800	1.007	80.714	375	1.006	80.412	375	
S15	80.596	1.772	Open	Manhole	1500	1.008	79.650	225	1.007	78.824	375	
S16	80.550	0.930	Open	Manhole	1500	1.009	79.620	225	1.008	79.620	225	

McCloy Consulting Limited		Page 5
Mossley Mill	M01616-36 Storm Rev 2	
Newtownabbey	Machaire BESS	
Co. Antrim		Micro
Date 10/06/2025 16:58	Designed by JD	
File M01616-36 MD new layout R2.mdx	Checked by IB	Drainage
Innovyze	Network 2019.1	<u>'</u>

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam.,L*W (mm)	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
S17	80.000	0.576	Open Manhole	0	OUTFALL		1.009	79.424	225	

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S1	296862.775	414322.451	296862.775	414322.451	Required	
S2	296859.894	414339.554	296859.894	414339.554	Required	ļ
S3	296849.618	414398.883	296849.618	414398.883	Required	I,
S4	296839.437	414458.666	296839.437	414458.666	Required	•
<b>S</b> 5	296879.570	414465.396	296879.570	414465.396	Required	

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Mossley Mill	M01616-36 Storm Rev 2				
Newtownabbey	Machaire BESS				
Co. Antrim		Micro			
Date 10/06/2025 16:58	Designed by JD	Drainage			
File M01616-36 MD new layout R2.mdx	Checked by IB	Diamage			
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MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S6	296991.324	414398.022	296991.324	414398.022	Required	1
s7	296981.850	414452.277	296981.850	414452.277	Required	-0
S8	296929.978	414352.942	296929.978	414352.942	Required	I,
S9	296923.066	414398.392	296923.066	414398.392	Required	ļ
S10	296916.105	414441.019	296916.105	414441.019	Required	ļ
S11	296968.422	414480.047	296968.422	414480.047	Required	_
S12	296911.201	414470.123	296911.201	414470.123	Required	
S13	296908.717	414487.220	296908.717	414487.220	Required	-

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Mossley Mill	M01616-36 Storm Rev 2					
Newtownabbey	Machaire BESS					
Co. Antrim		Micro				
Date 10/06/2025 16:58	Designed by JD	Drainage				
File M01616-36 MD new layout R2.mdx	Checked by IB	Dialilade				
Innovyze	Network 2019.1	,				

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S14	296962.504	414496.216	296962.504	414496.216	Required	
S15	296958.293	414523.397	296958.293	414523.397	Required	•

McCloy Consulting Limited	Page 8	
Mossley Mill	M01616-36 Storm Rev 2	
Newtownabbey	Machaire BESS	
Co. Antrim		Micro
Date 10/06/2025 16:58	Designed by JD	Drainage
File M01616-36 MD new layout R2.mdx	Checked by IB	Dialilacie
Innovyze	Network 2019.1	

### PIPELINE SCHEDULES for Surface Network 2

# <u>Upstream Manhole</u>

PN	Hyd Sect		MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
1.000	0	375	S1	83.250	82.225	0.650	Open Manhole	1800
1.001	0	375	S2	83.250	82.063	0.812	Open Manhole	1800
1.002	0	375	s3	83.250	81.762	1.113	Open Manhole	1800
1.003	0	375	S4	83.250	81.459	1.416	Open Manhole	1800
1.004	0	375	S5	82.730	81.256	1.099	Open Manhole	1800
2.000	0	300	S6	83.250	82.225	0.725	Open Manhole	1800
2.001	0	300	s7	83.250	81.783	1.167	Open Manhole	1800

### Downstream Manhole

PN	Length (m)	Slope (1:X)		C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
1.000	17.343	107.1	S2	83.250	82.063	0.812	Open Manhole	1800
1.001	60.213	200.0	s3	83.250	81.762	1.113	Open Manhole	1800
1.002	60.643	200.1	S4	83.250	81.459	1.416	Open Manhole	1800
1.003	40.694	200.5	S5	82.730	81.256	1.099	Open Manhole	1800
1.004	31.982	87.9	S12	83.000	80.892	1.733	Open Manhole	1800
2.000	55.076	124.6	s7	83.250	81.783	1.167	Open Manhole	1800
2.001	66.702	149.9	S10	83.250	81.338	1.612	Open Manhole	1800

McCloy Consulting Limited	Page 9	
Mossley Mill	M01616-36 Storm Rev 2	
Newtownabbey	Machaire BESS	
Co. Antrim		Micro
Date 10/06/2025 16:58	Designed by JD	Drainage
File M01616-36 MD new layout R2.mdx	Checked by IB	Dialilacie
Innovyze	Network 2019.1	

### PIPELINE SCHEDULES for Surface Network 2

### <u>Upstream Manhole</u>

PN	Hyd Sect		MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
3.000 3.001	0		S8 S9		82.300 82.070		Open Manhole Open Manhole	1800 1800
2.002	0	300	S10	83.250	81.338	1.612	Open Manhole	1800
4.000	0	225	S11	83.250	82.200	0.825	Open Manhole	1500
1.005 1.006	0	375 375	S12 S13	83.000 83.250	80.892 80.777		Open Manhole Open Manhole	1800 1800

### Downstream Manhole

PN	Length (m)	Slope (1:X)			I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
	45.973						Open Manhole	
3.001	43.191	200.0	S10	83.250	81.854	1.096	Open Manhole	1800
2.002	29.514	79.6	S12	83.000	80.967	1.733	Open Manhole	1800
4.000	58.075	150.1	S12	83.000	81.813	0.962	Open Manhole	1800
1.005	17.277	150.2	S13	83.250	80.777	2.098	Open Manhole	1800
1.006	54.534	149.4	S14	83.250	80.412	2.463	Open Manhole	1800
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McCloy Consulting Limited		Page 10
Mossley Mill	M01616-36 Storm Rev 2	
Newtownabbey	Machaire BESS	
Co. Antrim		Micro
Date 10/06/2025 16:58	Designed by JD	Drainage
File M01616-36 MD new layout R2.mdx	Checked by IB	praniacie
Innovyze	Network 2019.1	'

### PIPELINE SCHEDULES for Surface Network 2

# <u>Upstream Manhole</u>

PN	Hyd	Diam	MH	C.Level	I.Level	D.Depth	MH	MH DIAM., L*W
	Sect	(mm)	Name	(m)	(m)	(m)	Connection	(mm)
1.007	0	375	S14	83.250	80.714	2.161	Open Manhole	1800
1.008	0	225	S15	80.596	79.650	0.721	Open Manhole	1500
1.009	0	225	S16	80.550	79.620	0.705	Open Manhole	1500

# <u>Downstream Manhole</u>

PN	Length	Slope	MH	C.Level	I.Level	D.Depth	MH	MH DIAM., L*W
	(m)	(1:X)	Name	(m)	(m)	(m)	Connection	(mm)
1.007	27.505	14.6	S15	80.596	78.824	1.397	Open Manhole	1500
1.008	4.461	148.7	S16	80.550	79.620	0.705	Open Manhole	1500
1.009	29.443	150.2	S17	80.000	79.424	0.351	Open Manhole	0

McCloy Consulting Limited		Page 11
Mossley Mill	M01616-36 Storm Rev 2	
Newtownabbey	Machaire BESS	
Co. Antrim		Micro
Date 10/06/2025 16:58	Designed by JD	
File M01616-36 MD new layout R2.mdx	Checked by IB	Drainage
Innovyze	Network 2019.1	'

### Area Summary for Surface Network 2

-				Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
	User User User User User User User User	Name	100 100 65 100 100 65 65 100 65 65 100 100 100	0.092 0.172 0.281 0.000 0.000 0.277 0.170 0.244 0.115 0.097 0.179 0.000 0.000 0.000	0.092 0.172 0.183 0.000 0.000 0.180 0.110 0.159 0.115 0.063 0.116 0.000 0.000 0.000	0.092 0.172 0.183 0.000 0.000 0.180 0.110 0.159 0.115 0.016 0.000 0.000
1.009			100	Total 1.627	Total 1.190	Total 1.190

# Free Flowing Outfall Details for Surface Network 2

Out	fall	Outfall	C. Level	I. Level		Min	D,L	W
Pipe	${\tt Number}$	Name	(m)	(m)	I.	Level	(mm)	(mm)
						(m)		
	1.009	S17	80.000	79.424		0.000	0	0
		©1	982-2019	Innovy	ze			

McCloy Consulting Limited		Page 12
Mossley Mill	M01616-36 Storm Rev 2	
Newtownabbey	Machaire BESS	
Co. Antrim		Micro
Date 10/06/2025 16:58	Designed by JD	Drainage
File M01616-36 MD new layout R2.mdx	Checked by IB	pramage
Innovyze	Network 2019.1	

#### Online Controls for Surface Network 2

### Hydro-Brake® Optimum Manhole: S16, DS/PN: 1.009, Volume (m³): 1.8

Unit Reference	MD-SHE-0151-1060-0900-1060	Sump Available	Yes
Design Head (m)	0.900	Diameter (mm)	151
Design Flow (1/s)	10.6	Invert Level (m)	79.620
Flush-Flo™	Calculated	Minimum Outlet Pipe Diameter (mm)	225
Objective	Minimise upstream storage	Suggested Manhole Diameter (mm)	1200
Application	Surface		

Control Points	Head (m) Flo	ow (1/s)	Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	0.900	10.6	Kick-Flo®	0.628	8.9
Flush-Flo™	0.285	10.6	Mean Flow over Head Range	_	9.0

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow $(1/s)$	Depth (m) Fl	ow (1/s)	Depth (m)	Flow (1/s)						
0.100	5.4	0.600	9.4	1.600	13.9	2.600	17.5	5.000	23.9	7.500	29.1
0.200	10.4	0.800	10.0	1.800	14.7	3.000	18.7	5.500	25.1	8.000	30.0
0.300	10.6	1.000	11.1	2.000	15.4	3.500	20.2	6.000	26.1	8.500	30.9
0.400	10.4	1.200	12.1	2.200	16.2	4.000	21.5	6.500	27.2	9.000	31.8
0.500	10.1	1.400	13.0	2.400	16.9	4.500	22.8	7.000	28.2	9.500	32.5

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Newtownabbey	Machaire BESS				
Co. Antrim		Micro			
Date 10/06/2025 16:58	Designed by JD	Drainage			
File M01616-36 MD new layout R2.mdx	Checked by IB	pranade			
Innovyze	Network 2019.1				

### Storage Structures for Surface Network 2

Tank or Pond Manhole: S15, DS/PN: 1.008

Invert Level (m) 79.650

Depth (m) Area (m<sup>2</sup>) Depth (m) Area (m<sup>2</sup>)

0.000 470.0 0.946 730.0

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Innovyze	Network 2019.1	·			

#### Simulation Criteria

Areal Reduction Factor 1.000 Manhole Headloss Coeff (Global) 0.500 MADD Factor \* 10m³/ha Storage 2.000
Hot Start (mins) 0 Foul Sewage per hectare (1/s) 0.000 Inlet Coefficient 0.800
Hot Start Level (mm) 0 Additional Flow - % of Total Flow 0.000 Flow per Person per Day (1/per/day) 0.000

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 1 Number of Real Time Controls 0

#### Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 16.800 Cv (Summer) 0.750 Region Scotland and Ireland Ratio R 0.280 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0 DVD Status ON
Analysis Timestep 2.5 Second Increment (Extended) Inertia Status ON
DTS Status ON

Profile(s)

Duration(s) (mins) 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080

Return Period(s) (years)

Climate Change (%)

									Water	Surcharged	Flooded			Pipe	
	US/MH		Return	Climate	First (X)	First (Y)	First (Z)	Overflow	Level	Depth	Volume	Flow /	Overflow	Flow	
PN	Name	Storm	Period	Change	Surcharge	Flood	Overflow	Act.	(m)	(m)	(m³)	Cap.	(1/s)	(1/s)	Status
1.000	S1	15 Winter	100	+20%	100/15 Winter				82.760	0.160	0.000	0.22		34.6	SURCHARGED
	©1982-2019 Innovyze														

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Co. Antrim		Micro
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US/MH Level
PN Name Exceeded

1.000 S1

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Co. Antrim		Micro			
Date 10/06/2025 16:58	Designed by JD	Drainage			
File M01616-36 MD new layout R2.mdx	Checked by IB	Dialilade			
Innovyze	Network 2019.1	-			

PN	US/MH Name	Storm		Climate Change	First (X) Surcharge	First (Y)	First (Z) Overflow	Overflow Act.	Water Level (m)	Surcharged Depth (m)		Flow /	Overflow (1/s)	Pipe Flow (1/s)
				090	5 4 <u>1</u> 5 11 41 5		0.02220		()	<b>\</b> /	\ <i>,</i>	oup.	(=/ =/	(=/-/
1.001	S2	15 Winter	100	+20%	100/15 Summer				82.751	0.313	0.000	0.73		96.8
1.002	s3	15 Winter	100	+20%	100/15 Summer				82.637	0.500	0.000	0.99		130.4
1.003	S4	15 Winter	100	+20%	100/15 Summer				82.532	0.698	0.000	0.77		99.3
1.004	S5	15 Winter	100	+20%	100/15 Summer				82.426	0.795	0.000	0.57		109.0
2.000	S6	15 Winter	100	+20%	100/15 Summer				83.027	0.502	0.000	0.66		62.0
2.001	s7	15 Winter	100	+20%	100/15 Summer				82.908	0.825	0.000	0.80		69.2
3.000	S8	15 Winter	100	+20%	100/15 Summer				82.931	0.331	0.000	0.72		53.1
3.001	S9	15 Winter	100	+20%	100/15 Summer				82.837	0.467	0.000	1.10		80.1
2.002	S10	15 Winter	100	+20%	100/15 Summer				82.722	1.084	0.000	1.23		139.2
4.000	S11	15 Winter	100	+20%	100/15 Summer				82.478	0.053	0.000	1.05		42.8
1.005	S12	15 Winter	100	+20%	100/15 Summer				82.266	0.999	0.000	1.86		246.6
1.006	S13	15 Winter	100	+20%	100/15 Summer				81.868	0.716	0.000	1.60		243.5
1.007	S14	15 Winter	100	+20%					80.909	-0.180	0.000	0.53		244.3
1.008	S15	480 Winter	100	+20%	100/15 Summer				80.530	0.655	0.000	0.40		11.9
1.009	S16	480 Winter	100	+20%	100/15 Summer				80.531	0.686	0.000	0.27		10.6

	US/MH		Level
PN	Name	Status	Exceeded
1.001	S2	SURCHARGED	
1.002	s3	SURCHARGED	
1.003	S4	SURCHARGED	
1.004	S5	SURCHARGED	
0	1982-	2019 Innov	yze

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Co. Antrim		Micro			
Date 10/06/2025 16:58	Designed by JD	Drainage			
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Innovyze	Network 2019.1				

	US/MH		Level
PN	Name	Status	Exceeded
2 000	0.0	ELOOD DIGH	
2.000	S6	FLOOD RISK	
2.001	s7	SURCHARGED	
3.000	S8	SURCHARGED	
3.001	S9	SURCHARGED	
2.002	S10	SURCHARGED	
4.000	S11	SURCHARGED	
1.005	S12	SURCHARGED	
1.006	S13	SURCHARGED	
1.007	S14	OK	
1.008	S15	FLOOD RISK	
1.009	S16	FLOOD RISK	



# ANNEX C –M01616-36 DWG101-04 Revised Drainage Plan

